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The impact of initial imperfections on the fatigue assessment of tower flange connections in floating wind turbines: A review

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During the massive manufactures and installations of fixed offshore wind turbines in China, initial imperfections were often found in the inspection. As more and more attentions and efforts of the wind energy sector have been devoted to deep waters with fixed and floating wind turbines (FWTs), the impact of such initial imperfections on fatigue assessment is paramount to the reliable design and safe operation, which warrant rigorous study. This paper presents a comprehensive review of three different initial imperfections. A brief introduction on FWTs and flange connections is provided at first. This is followed by a detailed discussion of the environmental loadings and fatigue assessment on the flange bolted connections. Finally, a comprehensive review of the state-of-art research on three common initial imperfections, including flatness divergence, bolt loosening and tower inclination, are presented. Their impact on fatigue assessment is further discussed.

KEYWORDS

floating wind turbine, fatigue assessment, flatness divergence, bolt loosening, tower inclination, initial imperfections

1 Introduction

Offshore wind power industry in China is growing rapidly to pursue a net-zero future. In 2021, the capacity of Chinese offshore wind power increased by 16.9 gigawatt, more than any other country's increase. All these offshore wind turbines (OWTs) are fixed in shallow waters. With such a rapid expansion, the potential of shallow water wind

power is nearly exhausted. The offshore wind energy sector has been increasingly focusing on the relatively deep water with floating wind turbines (FWTs).

Different from fixed OWTs, FWTs are restrained by various mooring systems typically undergo displacement in six degreesof-freedom. They are subjected to wind, wave and current environmental loadings. The wave load is directly acting on the foundations or supporting structures. The upper structure is vulnerable to wind load and dynamic loading from blades' rotations. The intermediate towers connect the upper structure and foundations, which are often subjected to multiple cyclic loadings from different sources. Hence, it has been often reported on fatigue damage in the tower of an operating wind turbine (Zou et al., 2021; Gao et al., 2022; Zheng and Chen, 2022; Wang et al., 2022).

To assess the fatigue damage on FWTs, Kvitten and Moan (Kvittem and Moan, 2015) presented a set of time-domain analysis procedures. An aero-hydro-servo-elastic tool FAST was used to simulate the global response of FWTs. The authors investigated the effect of simulation duration, number of random realisations and bin sizes to obtain the distribution of cyclic hot spot stress over the tower sections. Luan et al. (Fang et al., 2020; Fang et al., 2021) developed Kvitten's approach and extended its capabilities to determine the global forces and moments in FWTs' components through a beam element model. The result of Luan's approach can output the time-series of stress over tower sections, which is the boundary conditions for following local model (flange connections)

analysis. Xu et al. (Xu et al., 2019; Xu et al., 2019) upgraded Kevitten's approach with fully nonlinear wave theory to investigate the effect of wave nonlinearity on FWTs' fatigue damage and extreme response. They concluded that effect of wave nonlinearity is significant for FWTs' surge motions and not sensitive to tower base bending moments.

During the large number of OWTs' installations in China, a lot of imperfect geometric conditions are found in the wind turbines' towers. Due to improper manufactures and installations, different initial imperfections may occur and undermine the fatigue strength of wind towers, see Figure 1. To improve the design of FWTs in the future exploitation of deep waters, the impact of initial imperfections on the fatigue assessment of FWTs' towers should be clarified.

2 Floating wind turbines and flanges

The components of FWTs include upper structure (rotornacelle assembly), substructure (tower and foundation) and mooring system. In the past decades, significant research focused on the global response simulations of FWTs (Dong et al., 2012; Kvittem and Moan, 2015; Luan et al., 2017; Luan et al., 2018; Xu et al., 2019), as it is the foundation for any other further analysis. It is still challenging to simulate the global motion of FWTs accurately, because FWTs are flexible and subjected to wind, wave and current simultaneously, which makes FWTs' motions rather complex. Hence, an aero-hydro-



servo-elastic analysis is required to simulate the coupled effect of environmental loadings on the structural motions. Many numerical programs have been developed to simulate the global response of FWTs, as listed in Table 1.

Wind turbine failure is often due to the failure of rotor blades or of the steel tower (Márquez et al., 2016).Wind turbines' towers are typically manufactured in steel segments with bolted flange connections. Due to the stress concentration around the flange, many fatigue cracks have been found around the flange connections of the inland wind turbines, see Figure 2. Offshore FWTs suffer from more severe environmental loadings, thus more attentions should be paid on their design of flange connections (Mehmanparast et al., 2020).

The conventional bolted flanges are L-shaped, as seen in Figure 3. Researchers have extensively investigated the stiffness and mechanical resisting capacities of conventional L-shape flanges (Seidel, 2001; Heistermann et al., 2009; Schwingshackl and Petrov, 2012; Van-Long et al., 2013; Wang et al., 2013). When wind turbines are installed increasingly from shallow water into deep water, the thickness and diameter of wind turbine towers are also increasing to support larger rotors and nacelles accordingly. Another type of flanges are Reverse Balanced (RB) flanges, see Figure 3. The RB flange connections consist of flange plates, stiffening plates, and

high-strength blots. Compared to the conventional bolted flanges, RB flanges have advantages such as higher stiffness and lower cost (He et al., 2016).

During manufactures and installations, imperfect geometric flange connections may occur which can lead to initial imperfections such as flatness divergence or tower inclinations. In addition, the bolt loosening may also undermine the fatigue strength of flange connections. The details of these initial imperfections and their impacts on fatigue damage are discussed in Section 7.

The design of offshore wind turbines requires both functionality and safety. For safety purpose, the OWTs should be strong enough under ultimate limit states (ULS), fatigue limit states (FLS) and accidental limit states (ALS). Among them, FLS is especially important for FWTs, because the system is subjected to millions of cycles of dynamic loads during the whole service life of FWTs.

According to the experience from inland wind turbine, many wind turbine collapse cases are due to the fatigue damage on cylinder tower and flanges (Alonso-Martinez et al., 2019). In order to ensure the safety of OWTs, the environmental loads, including wind, wave and current, should be clarified first (Hu et al., 2020). All these environmental loads induce aerodynamic and hydrodynamic impacts on the structures.

TABLE 1 Numerical programs for the analysis of floating wind turbines' global response.

Aerodynamic analysis	Hydrodynamic analysis	Time-domain simulation of WT					
Turbsim	Sesam (HydroD/Wadam)	Sesam (SIMA/SIMO+REFLEX)					
IEC wind simulator	WAMIT	OpenFast					
	Nemoh	HAWC2					
		Bladed					



Fatigue cracks and fatigue failures of inland wind turbine towers (Zou et al., 2021).



2.1 Wind

The description of wind can be divided into two parts: spatial part and temporal part, as seen in Equation (1)

$$U = \{u_s(x, y, z) + [u_1(x, y, z, t), u_2(x, y, z, t), u_3(x, y, z, t)]\}$$
(1)

where $u_s(x,y,z)$ denotes the mean wind speed spatial variation, and $[u_1(x,y,z,t), u_2(x,y,z,t), u_3(x,y,z,t)]$. stands for the temporal variation. The spatial mean wind speed variation is usually described by wind shears, and the temporal variation is

represented by wind turbulence in vertical, longitudinal and lateral directions. In addition, the temporal variation can also be represented by frequency spectrum models such as Kaimal or Mann models. The temporal variation is described as stationary or non-stationary depending on the time scales. In practice, a short-term wind condition with 10 minutes period is assumed as stationary.

For OWTs, wind conditions are the primary external environmental conditions, while the wave and current conditions may also play a role. The aerodynamic loads are

dependent (among other factors) upon the rotational speed of the rotor, the average wind speed across the rotor plane, the three-dimensional turbulence intensity, the wind shear, wind direction changes, the density of the air, the aerodynamic shapes of the wind turbine components and their interactive effects, including the aeroelastic effects (DNVGL (2016a)). According to the blade element momentum theory, wind blows over airfoil and generates lift force and drag force. These forces rotate the blades and push the wind tower structure in horizontal direction, which eventually results in torque and thrust acting on the wind tower. The other loads acting on the wind tower include the wind-induced pressure, the eccentric moment from the gravity of nacelle and blades. The OC3 baseline 5 MW NREL offshore wind turbine is exemplified here with rated wind speed 11.4 m/s, rotor diameter 126m, mean torque 2500 KN• m and mean thrust 300KN (Goupee et al., 2017).

2.2 Wave

The distribution of ocean waves is usually described by wave spectra. Depending on the location and the sea state, wave spectra contain substantial energy mainly in the range from 5 to 25 seconds. For a FWT, the natural periods of motion should be designed to avoid resonance. The natural period in heave is normally above 25 second for a spar and below 5 second for a tension leg platform. The short-term wave conditions are usually described as stationary with 1-3 hours, and they are characterized by wave height and wave period. For instance, JONSWAP spectrum is used to describe its frequency distribution of wave energy for fetch-limited seas; Pierson-Moskowitz (PM) spectrum is for fully developed seas. Sometimes, a two peak spectrum is used to describe combined seas with wind wave and swell. To simplify the calculations, a linear wave assumption is often applied, but the nonlinear effect of wave is getting more and more attentions these years (Xu et al., 2019; Xu et al., 2019). Stationarity of wave conditions is usually assumed for 1-3 hours.

The hydrodynamic forces on slender structures are usually calculated based on Morison's formula, which is a semiempirical formulation to capture the wave force on a fixed or floating offshore structures. It's composed of three terms to stand for inertia force, drag force and added mass force induced by the structures' acceleration in water. For large volume structures, linear potential theory is recommended to calculate the diffraction and radiation forces. Computational Fluid Dynamics (CFD) approach is utilized to mathematically model a physical-fluid-flow related phenomenon (Gao et al., 2020; Gao et al., 2021). CFD approach is used to solve strongly nonlinear problems. It has the highest accuracy if properly verified and validated but can be computational expensive.

Current may also affect the FWT's motions especially on supporting structures of extensive draft, but it mainly acts on the

mooring systems and contributes less to fatigue damage on the towers. Therefore, current force is not discussed here.

3 Fatigue assessment

Fatigue damage is a cumulative process and it requires longterm sea condition data to predict. Unlike structural failures due to insufficient ultimate strength, fatigue failures usually attribute to moderate sea conditions, because the occurrence of moderate sea conditions is much higher than that of extreme conditions. The long-term sea conditions are usually represented either by a longterm distribution of several sea state related parameters or the sum of short-term sea conditions. A structure's fatigue design should ensure that its fatigue strength can resist all these wave conditions in the operation stage. However, initial imperfections may introduce stress concentration and undermine the fatigue strength of flange connections. They should be considered in the design stage.

3.1 S-N curve based approach

For offshore structures' design, fatigue damage is usually calculated based on the S-N curve approach. The fatigue resistance is represented by S-N curves. If the long-term stress range distribution is defined as the sum of the Rayleigh distribution of each short-term stress range corresponding to each sea state, the cumulative fatigue damage with one slope S-N curve is given by:

$$\mathbf{D} = \frac{\omega_0 T_d}{a} \Gamma \left(1 + \frac{m}{2} \right) \sum_{n=1}^{N_{load}} p_n \sum_{i=1,j=1}^{n} r_{ijn} \left(2\sqrt{2m_{0ijn}} \right)^m \quad (2)$$

where D is accumulated fatigue damage, ω_0 is the average long-term zero-crossing frequency, *a* and m are the S-N curve parameters, T_d is the design life of offshore structures Γ is gamma function, N_{load} is total number of load conditions, p_n is the fraction of design life in load condition *n*, r_{ijn} is relative number of stress cycles in short-term condition *i*, *j*, m_{0ijn} the zero spectral moment of stress response process.

The stress response spectrum and spectral moments in linear models are defined as

$$S_{\sigma}(\omega|H_s, T_z, \theta) = |H_{\sigma}(\omega|\theta)|^2 \cdot S_{\eta}(\omega|H_s, T_z)$$
(3)

$$m_n = \int_{\omega} \sum_{\theta=90^{\circ}}^{\theta+90^{\circ}} f(\theta) \omega^n \cdot S_{\sigma}(\omega | H_s, T_z, \theta) d\omega$$
(4)

where $H_{\sigma}(\omega|\theta)$ is the transfer function representing the relation between unit wave amplitude and response, $S_n(\omega|H_s, T_z)$ is wave spectrum, $f(\theta)$ is the wave spreading function. It can

be seen that, for a particular offshore structure, the fatigue damage is mainly determined by sea states which are represented by wave scatter diagrams, as seen in Table 2. The blocks with dark color background contribute most to the fatigue damage, because they have much higher frequency of occurrences.

3.2 Full long-term analysis

As discussed in section 3.2, wave loadings are the primary source of fatigue damage for offshore structures. In design stage, it is highly recommended to identify all the wave conditions which the offshore structures will encounter in their service life.

Wave scatter diagram combines all effective short-term extreme response distribution associated with their corresponding probability of occurrence. Each block is defined as the statistical description of wave conditions over 3 or 6 hours with its significant wave height (H_s), zero crossing period (T_z) and occurrence. The sea states in scatter diagrams are characterized based on observations, measurements or hindcast. Then, the fatigue damage in each shortterm sea state is calculated, and the long-term fatigue damage is accumulated by the summation of all short-term fatigue damages.

3.3 Global-local methodology

In this section, a global-local methodology of the fatigue calculation procedure is described, emphasizing the theoretical concepts associated with the modal superposition and fatigue methods. A general workflow for the global-local fatigue assessment methodology is suggested in Figure 4, introducing two different phases of analysis that follow an increasing detail of geometrical, material and contact properties as the assessment progresses from the global to the local scale. The two fatigue analysis phases are systematized as follows. In the first phase, a global approach based on the accumulation of damage computed with nominal stresses and S-N curves is introduced. The adopted global numerical model should be accurate enough to obtain nominal stresses to be used as input loading for the linear damage accumulation method. Nonetheless, the available S-N curves with nominal stress may not properly reflect the local geometrical and material characteristics of critical details, and conservative assessment for the fatigue resistance may be considered. Based on this fatigue analysis result, the critical details should be identified which require a more refined assessment by implementing local scale models. Then, in the second phase, submodelling techniques leveraged by modal superposition concepts are used to evaluate accurately local fatigue damage. Hot spot stress and notch stress concepts are proposed to consider more geometrical and local surfaces details (Horas et al., 2022).

4 External loadings on flange bolted connections

According to the section above, wind towers are subjected to various external dynamic and constant loadings. Some of them are

		Tz(s)																
		3.5	4.5	5.5	6.5	7.5	8.5	9.5	10.5	11.5	12.5	13.5	14.5	15.5	16.5	17.5	18.5	sum
Hs(m)	0.5	1	134	866	1186	634	186	37	6	1	0	0	0	0	0	0	0	3050
	1.5	0	29	986	4976					161	31	5	1	0	0	0	0	22575
	2.5	0	2	198	2159						160	34	6	1	0	0	0	23810
	3.5	0	0	35	696	3227						84	18	4	1	0	0	19128
	4.5	0	0	6	196	1354						131	32	7	1	0	0	13289
	5.5	0	0	1	51	498	1603					151	41	10	2	0	0	8328
	6.5	0	0	0	13	167	690					141	42	11	3	1	0	4806
	7.5	0	0	0	3	52	270	594			277	112	37	10	3	1	0	2586
	8.5	0	0	0	1	15	98	256	351	297	175	78	28	8	2	1	0	1309
	9.5	0	0	0	0	4	33	102	160	152	99	48	19	6	2	0	0	626
	10	0	0	0	0	1	11	38	68	72	52	27	11	4	1	0	0	285
	11.5	0	0	0	0	0	3	13	27	31	25	14	6	2	1	0	0	124
	12.5	0	0	0	0	0	1	4	10	13	11	7	3	1	0	0	0	51
	13.5	0	0	0	0	0	0	1	4	5	5	3	2	1	0	0	0	21
	14.5	0	0	0	0	0	0	0	1	2	2	1	1	0	0	0	0	8
	15.5	0	0	0	0	0	0	0	1	1	1	1	0	0	0	0	0	4
	sum	1	165	2091	9280	19922	24879	20870	12898	6245	2479	837	247	66	16	3	1	100000

TABLE 2 The wave scatter diagram.



acting directly on the wind tower, the rest are acting on nacelles, blades and substructures. For fatigue assessment of flange bolted connections, the sensitive areas, where high geometric stress concentration occurs, should be clarified first. There are basically two types of sensitive area in the tower: bolts and welding seams. The local structure of flange bolted connections and sensitive areas are shown in Figure 1. The external loadings over the tower cross section are classified as follows:

- 1. The torque induced x-axis moments M_x
- 2. The wind thrust induced y-axis moments M_{v1}
- 3. The wind pressure induced y-axis moments M_{v2}
- 4. The wind induced shear S_1
- 5. The by the eccentric gravity of upper structure (rotor and nacelle) G1 and its induced y-axis moments M_{y3}
- 6. The gravity of tower itself G_2
- 7. The FWT's motion induced inertia forces F(i)

In short, the flange connections are mainly subjected to moments, pressure and shear. The axial stress over the tower cross section was calculated according to Equation (5). The shear is neglected due to its minor contribution to fatigue damage.

$$\sigma = \frac{G}{A} + \frac{M_y(t)}{I_y} r_1 \cos\theta - \frac{M_x(t)}{I_y} r_1 \cos\theta$$
(5)

$$G = G_1 + G_2 \tag{6}$$

$$M_{\nu} = M_{\nu 1}(t) + M_{\nu 2}(t) + M_{\nu 3} \tag{7}$$

 M_{y3} and gravity loadings are considered as constant loadings. They do not affect the stress range over the cross section, but may still play a role in fatigue assessment by increasing or decreasing the mean stress. For FWTs, the motion induced inertia forces should be considered.

As FWTs' substructures are not fixed to the seabed, they have oscillating motions in six degrees of freedom (DOFs). The substructure's motions may influence the aerodynamic condition of rotor, which makes the aerodynamic loadings on FWTs much more complicated than those on fixed wind turbines. Mostafa et al. (Nihei et al., 2012) studied the interactions between the rotary motion of the wind turbine blade and the dynamic motion of the Spar type FWT through both numerical simulations and experiments. Among all motions in DOFs, surge and pitch are regarded as the most important ones affecting the FWTs' aerodynamics (Bayati et al., 2016; Fang et al., 2020; Fang et al., 2021). Once the aerodynamic conditions are affected by the substructures' motions, the external forces on the wind tower, such as M_x and M_{v1}, are also changed. Hence, it is necessary to consider the impact of FWTs' motions in the fatigue assessment. This impact is further discussed in section 7.3.

5 Numerical model of flange connections

The research on fatigue assessment for wind turbine towers requires experiments (Shirani and Härkegård, 2011a; Shirani

and Härkegård, 2011b; Van-Long et al., 2013; Schaumann and Eichstädt, 2015; Oechsner et al., 2015) or on-site measurements (Pollino and Huckelbridge, 2012; Benedetti et al., 2013). However, due to the limit of these measured data, numerical modelling is more widely used to analyze the fatigue life of local flange connections. In a numerical model of large flange connections, the fatigue strength involves many factors which should be considered (Sutherland, 1999; Ding and Chen, 2013; Do et al., 2015; Do et al., 2015; Ragan and Manuel, 2016; Blachowski and Gutkowski, 2016). The large size of flange connections in OWTs means large dimensions, large number of bolts and more complex interactions, which makes more uncertainties in modellings (Redondo and Mehmanparast, 2020). Hobbs et al. (Hobbs, 2000) have investigated the fatigue strength of M12 high tensile bolts. Their study revealed that bolted joints' fatigue strength is highly related to the stiffness level of joints and the preload level. Couchaux et al. (2017) pointed out that secondary forces are induced in bolts by the contact stress between flange parts.

The finite element method (FEM) is a recommended approach to analyze the stress distribution around flange connections, as shown in Figure 5. To simplify the modelling and minimize the computational cost, one or a half flange segment of interest is usually generated through ANSYS, ABAQUS or other FEM software. There are three key issues in the FEM modellings: element selection and mesh, external load and boundary conditions, preload and fiction between contact surfaces.

A finite element model should not only have a low computational requirement but reflect the key characteristics such as irregular geometries and large deformations. Finite elements with more nodes, for example SOLID95 in ANSYS, are often recommended to model the structural parts with irregular geometries or large deformations; finite elements with less nodes, for example SHELL93 in ANSYS, for other parts. Besides, a pretension element is used to apply preload on the bolts. As for meshing, a small mesh size is used to mesh the areas with high stress concentration; while relatively large mesh size is adopted for the other areas.

In the fatigue assessment of wind tower, cyclic loadings on the tower shell sections are simulated by the global simulations and transferred as the boundary conditions for the local model simulation. Then, a local model of flange is required to obtain the stress time series at the sensitive locations. A load transfer function (LTF) is usually used to calculate the cyclic loadings at the bolts or welding seams based on the boundary conditions.

The FE model is under compression or tension depending on where this segment located around the ring flange. The lower surface of tower shell is usually fixed as the boundary condition. The whole tower shell also suffers from wind load, and a preloading force is applied on the bolt and flange. All these loads are shown in Figure 5.

The preload on the pretension elements connects the upper and lower parts of flanges together. DNVGL ST-0126 and DNVGL-OS-C101 have suggested the requirements for the bolt preloading (DNVGL (2018); DNVGL (2016b)).

$$F_p \le 0.7 f_{ub} A_s \tag{8}$$

where:

 F_p = preloading force

 f_{ub} = ultimate tensile strength of bolt material

 A_s = stress area of bolt

However, there are many uncertainties exist in the preloading forces during the installation stage. Attention should also be paid on the self-loosening effect, which will be further discussed in Section 7.2. The contact surface between the flange parts is set as frictional; and the contacts between the bolts and flanges are often defined as either frictional (Alonso-Martinez et al., 2019; Weijtjens et al., 2021) or bonded (Badrkhani Ajaei and Soyoz, 2020), see Figure 6. The frictional



contacts are defined with frictional coefficients μ Ajaei and Soyoz recommended a value of the frictional coefficient between two flange parts as 0.45 according to DNV-OS-C101 (DNVGL (2016b)). Gorst et al. (Weijtjens et al., 2021) suggested μ =0.25 for contacts between washers and flange surfaces.

6 Initial imperfection

Flange imperfections have a significant influence on the LTF (Weijtjens et al., 2021). Since the design criterion of FWTs is higher than that of fixed wind turbines, it is necessary to pay more attention on the occurrence of initial imperfections during the manufacture and installation of FWTs. China's offshore industry has manufactured and installed significant number of offshore wind turbines during recent years. Among them, it is reported that three types of initial imperfections are most common to occur. They are flatness divergence, bolt loosening and tower inclination.

6.1 Flatness divergence

Under perfect geometric conditions, as the surface pressure between the two flange parts increases due to the tightening, a clamp force emerges centrically around the longitudinal bolt axis as a counterpart of the preload, as shown in Figure 7. With the clamp force distributed symmetrically around the bolt axis, external forces acting on the tower shell first diminish the clamp force before stressing the bolt. However, under imperfect flatness divergence conditions, the flange contact surface is not ideally plane due to a flange inclination α_{s} , a flange gap length *k* and a flange opening length *L*. The two flange parts may open, and the clamp force may emerge eccentric from the bolt's longitudinal axis when preloading. According to the location of openings, flatness divergence can be classified as inner sided, outer sided and parallel, as shown in Figure 8. When preloading, these gaps close initially, and then clamp solids emerge. Seidel (2018) investigated the fabrication tolerance of flange connections in wind turbine towers. His analysis shows





that the gap length and tower wall thickness are the driving factors to affect the gap closing behaviors. It is more difficult to close those gaps with shorter length and thicker tower walls by preloading. This conclusion is also confirmed by Lüddecke et al. (Lüddecke et al., 2019). Jakubowski (Jakubowski and Schmidt, 2003) analyzed the flanges through experiments with these three different opening types, The result shows that it is most difficult to close openings with parallel gaps, while the inner sided gaps appeared to be less difficult. The outer sider gaps, in contrast, play a positive role in LTFs.

DIN 18088-3 (DIN (2019)) as well as the DNVGL-ST-0126 (DNVGL (2018)) have stated the requirement for imperfect flanges. The flatness divergence of one flange must be less than 2 mm in length over the entire flange circumference and less than 1 mm over a 30° segment. However, Weijtjens et al. (2021) pointed out that this requirement is semi-empirical and it does not consider those



geometric imperfection explicitly. With the widely use of large flange and bolt diameters, it becomes more and more challenging to fulfil all these requirements during the OWT installations.

The flatness divergence may significantly influence the stress distribution over the whole flanges when external loading is acting on the tower. According to the standard (DASt, (2013)), load transfer functions are recommended to describe the relationship between external force Z and bolt load Fb. With the increase of external forces, this process can be described in four stages:

- 1. Stresses between the flanges are dominant by bolt preload, and the contact is closed.
- 2. Stresses increase and some small imperfection-induced openings show up. In this case, the clamp solid begins to degrade.
- 3. Stresses keeps on increasing and the openings become large.
- 4. Flanges and bolts begin to deform plastically.

For fatigue assessment, the first stage account most for fatigue life. The other stages are relatively less important. When clamp solids emerge, the stress may distribute differently depending on the geometric perfection or imperfection. Figure 9 illustrates that imperfect flange with flatness divergence has higher bolt load and variance which is more vulnerable to fatigue damage.

Schaumann and Seidel (2002) measured the stresses in the ring flanges of an onshore wind turbine. The result indicates that there is a strong fluctuation of load transfer functions due to the influence of geometric imperfections. Another research (Weijtjens et al., 2021) found the existence of flatness

divergences, but all of them satisfy the requirement of standards. FE models were built and their result was compared with the measurement. The comparison revealed that it is necessary to consider the effect of flatness divergence to match the FE analysis result with on-site measurements.

6.2 Bolt loosening

Ring-shaped flanges are widely used to connect the tubular steel segments of offshore wind turbine towers. With the high demand of large capacity generators, the bolts' size has grown to M36 or even bigger (Braithwaite and Mehmanparast, 2019). For safety purpose, high preload forces are applied on flanges by tightening bolts to keep flange connections close and strong. However, considerable observations have been recorded that the flange connections may turn loose during installations and operations (Badrkhani Ajaei and Soyoz, 2020). The bolt loosening reduces the clamp forces between flange parts and lowers the strength of connections (Wegener et al., 2020).

According to standards and guidelines (DNVGL (2016b); IEC, 2005; GL, 2010), all bolts in a flange should be preloaded uniformly. However, in the installation stage, this requirement is hardly fully satisfied due to the elastic interaction between nearby bolts or flanges. Tightening a bolt may affect the other tightened bolts and reduce their preloading forces. One practical solution is to tighten all these bolts in a scientific sequence with a limited number of passes to minimize the variation of their preloading forces (Tsuji and Nakano, 2002; Abid and Nash, 2006; Nassar et al., 2010; Braithwaite and Mehmanparast, 2019). Another reason of un-uniform preloading is that the accurate on-site measurement of preloading forces is still a challenge to engineers. Meisterling (2010)



monitored the preload forces of a series of M36 bolts, and found that the standard deviation of these bolts preload forces is more than 12%. Nagata et al. (2006) measured the bolt preloading forces in a vessel and indicated that the scatter of the preloads in the bolts of a flange can reach 45.2% of the intended preload. In addition, self-loosening of bolts is often found during OWTs' operations. It has been widely accepted that preloaded bolts may loosen themself when subjected to long term vibrations or cyclic loadings. Cyclic loadings are the source of fatigue damage. When they loosen the preloaded bolts and influence the cyclic stress fluctuations, OWT towers are more vulnerable to fatigue failures.

To investigate the effect of bolt preload level on fatigue strength of ring shaped flange connections, Ajaei and Soyoz (Badrkhani Ajaei and Soyoz, 2020) performed the fatigue assessment on a finite element model of the maximal-loaded segment of the bolted connection in an on-shore wind turbines. The flanges are connected by 88 M36 bolts with a designed preload level of 510kN. In order to simulate the process of bolt loosening, five preload levels are applied as 510kN, 255kN, 127.5kN, 51kN and 10⁻⁶ kN. In the numerical model, the upper edge is applied with external loadings (axial forces and bending moments), and the lower edge is fixed. The time series of bolt stress is shown in Figure 10. The

fatigue damage with different preload levels was calculated based on S-N curves. This research revealed that the reduction of bolt preload level from 510kN to nearly 0kN can cause an increase of the cyclic bolt stress ranges and result in more fatigue damage. In addition, the bending stresses in higher preload levels contribute much more to fatigue damage than in lower preload levels.

6.3 Tower inclination

Initial imperfections in flanges or improper installations of wind turbines may lead to tower inclination of fixed OWTs. DNVGL-ST-N001 (DNVGL (2016c)) requires that the relative motion of tilt is 2° in the lifting operation of upper modules. DNVGL-ST-0126 (DNVGL (2018)) states that the total tolerance for the tower axis tilt is 0.25°. However, the research on the tower inclination induced by initial imperfections of flange is rather limited, because its impact is much less significant than inclinations from other sources. For fixed offshore wind turbines, the surrounding seabed soil is losing strength and stiffness under cyclic external loadings and scour, and the tower inclination is mainly due to the soil-pile interface behaviors (Zhou et al., 2021).



(A) Time series of bolt stress with averages removed under different preloads; (B) Time series of bolt stress under different preloads; (C) stress range histograms; (D) fatigue damage under different preloads for one minute loading (Badrkhani Ajaei and Soyoz, 2020).

Tower inclinations for FWTs are, however, inevitable due to their floating foundations. FWTs typically experience motions from six DOFs as surge, sway, heave, roll, pitch and yaw (Jonkman, 2007; Jonkman, 2009). Considerable research has been conducted for the impact of motions on the aerodynamic and hydrodynamic performance of a FWT, including surge (Hu et al., 2015; Micallef and Sant, 2015; Tran and Kim, 2016), heave (Khosravi, 2015), pitch (Jeon et al., 2014; Hu et al., 2016; Wen et al., 2018; Wen et al., 2018; Ortolani et al., 2020), yaw (Miao et al., 2016) and the combined motion like pitch & yaw (Tran and Kim, 2015), pitch & surge (Bayati et al., 2016) or even surge, heave & pitch (Liu et al., 2016). Some research concluded that up to 10° inclination is acceptable for FWTs (Sebastian and Lackner, 2012a; Rockel et al., 2014; Antonutti et al., 2016).

Among all the motions, pitch and surge are believed to have the most significant impact on FWT's aerodynamic performance by different ways, as listed in Table 3. Fang et al. (Fang et al., 2020; Fang et al., 2021) validated their numerical model of a 1:50 scale FOWT

by experiments, and investigated the change of rotor's aerodynamics including thrust, torque and rotor power, which may lead to cyclic loadings on the flange connections and result in fatigue damage. Their comparisons indicates that pitch and surge motions with higher amplitude can result in higher range of thrust and torque variations, as shown in Figure 11. Antonutti et al. (2016) conducted study focusing on the effect of large inclination on the hydrodynamic performance of a FOWT and concluded that large inclinations can amplify the motions of FOWTs. The authors further pointed out that such impact is also very sensitive to the relative directions of wind and waves.

7 Conclusions

This paper provides a comprehensive review of the initial imperfections' impact on the fatigue assessment of FWT towers' flange connections. Among the massive recent installations of

TABLE 3 The impact of pitch and surge on aerodynamics of FWTs.

Pitch	By changing the relative velocity between air flow and rotors (Bayati et al., 2016) By changing the flow field state around rotors (Sebastian and Lackner, 2011; Sebastian and Lackner, 2012a; Rockel et al., 2014) By generating turbulence in the wake field (Hu et al., 2015; Khosravi, 2015).
Surge	By changing the relative velocity between air flow and rotors (Farrugia et al., 2014; Liu, 2019) By moving rotors in or out of the wake field (Sebastian and Lackner, 2012b)



OWTs in China, three types of initial imperfections were reported most likely to occur. They are flatness divergence, bolt loosening and tower inclination. As the global wind power industry tends to develop progressively towards deepwater offshore with FWTs, the impact of initial imperfections can be even more critical thus further studies in the topic are is warranted and timely.

The initial imperfections are likely to occur during manufacture and installations of FWTs, which has a negative impact on the fatigue strength of bolted flange connections. This paper reviewed recent literature about the impact of these initial imperfections. The main conclusions are as follows:

- 1. Flatness divergence opens the upper and lower flange parts and results in imperfect geometric conditions. The clamp force may distribute eccentrically from the bolt's longitudinal axis when preloading, and the stress fluctuations in the hotspots is highly dependent on the imperfect geometric conditions. The flatness divergence makes the flange connections more vulnerable to fatigue damage than under perfect conditions.
- 2. Large size bolts in the flange connections may lose preloading forces due to the vibration during the installation or operation stages. The self-loosening of preload forces may lead to an increase of cyclic stress range and result in a reduction of fatigue life.
- 3. Standards and guidelines have strict requirements for the tower tilt angle of fixed OWTs. However, due to the floating substructures, it is impossible to require FWTs to limit their tower tilt as much as fixed wind turbines. The research on FWTs' inclination reveals that the pitch and surge motions may enlarge the variation of torque and thrust, which also has a negative impact on the fatigue strength of flange connections.

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Author contributions

TZ: Conceptualization, Methodology, Software, Investigation, Formal Analysis, Writing - Original Draft XN: Picture Making XJ: Picture Making ML: Validation, Corresponding author LT: Writing - Review & Editing. All authors contributed to the article and approved the submitted version.

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Conflict of interest

The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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