

Reliability Modeling of Infrastructure Load-Sharing Systems with Workload Adjustment

Qiuzhuang Sun, Zhi-Sheng Ye, Matthew Revie, and Lesley Walls*

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Abstract

Motivated by the need to support effective asset management of infrastructure systems, this paper presents a novel reliability model for a load-sharing system where the operator can adjust component work load to balance system degradation. The operator intervention effect, combined with other system complexities, makes modeling reliability interesting and challenging. We first develop cost modeling for a load-sharing system that has experienced operational service at the time of analysis. The system replacement process is modeled as a delayed renewal process for which the expected operational cost of the system is derived. A numerical algorithm is proposed to compute the cost, and the error bound is shown to be of order $O(n^{-1})$. Next, we extend modeling to consider multiple heterogeneous systems located at different sites within the infrastructure network. Heterogeneities here refer to possible cross-site differences in the operating environments and the operators' actions. When the heterogeneities are observable, we model as covariates; otherwise, we model as random effects. Statistical inference methods are developed for the proposed models. An example using real data from a water utility illustrates the logical model behavior given parameter choices as well as showing how analysis might inform asset management.

Keywords: Delayed renewal process, degradation failure, cumulative work load, Gamma process, load-sharing system.

Acronyms

RGF	Rapid gravity filter
CWL	Cumulative work load
RS	Riemann-Stieltjes
SF	Survival function

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PDF	Probability density function
CDF	Cumulative distribution function
IG	Inverse Gaussian
MLE	Maximum likelihood estimator

Notations

T	Time to degradation failure of the system
X_i	CWL-to-degradation failure of component i
S	CWL-to-degradation failure of the system
$U(t)$	CWL to the system at t
N	Number of components in a system
z	Time that system has been in operational service at $t = 0$
u	CWL at $t = 0$
y	Annual average turbidity
$C(\tau)$	Expected cost over mission time τ
μ	Mean of the IG distribution for X_i
λ	Shape parameter of the IG distribution for X_i
γ	Scale parameter of accumulation of the CWL process
v	Shape parameter of accumulation of the CWL process
C_O	Operational cost of a system component
C_r	Replacement cost of a system component
C_s	Fixed system replacement cost

1 Introduction

An infrastructure system, such as a wind farm, water treatment works or power station, can consist of multiple functionally-identical components that synergistically work together and share the total load to the system. In order to achieve reliable system functioning, components are designed to be highly reliable in the sense that they seldom suffer from sudden failures. Nevertheless, components might degrade gradually with usage. Degradation in the condition or performance of a component can be measured using sensors, e.g., vibration frequency of a machine [1], wear particles in the lubrication oil [2], and the erosion level of an induction furnace [3]. Without accurate measurement, degradation can also be perceived visually, e.g., pitting corrosion on a turbine blade and clustering of the sand in rapid gravity filters of a water filtration process. A component is replaced when the degradation level is deemed unacceptable, which is commonly known as a degradation failure [4, 5]. A review on degradation analysis can be found in [6].

For the infrastructure systems of interest, the number of components is determined to ensure the system is capable of handling peak demand. However, during off-peak periods the reduced demand means not all components may be required to operate and the system work load is lowered. A rational strategy is to selectively not operate components with the highest degradation levels in order to balance the chronological lifetimes of all

components. Using this work load adjustment strategy implies that the degradation levels of all components are managed and maintained at a similar level. Specifically, we have observed this degradation management strategy for rapid gravity filters (RGFs) which are used during the water filtration process in water treatment works. For RGFs, as well as other critical components, the water utility have adopted this approach for several reasons. First, the fixed replacement cost is high relative to the variable cost. Therefore, by managing the degradation of all RGFs used within a water filtration process, the performance of these components can be kept approximately at the same level, and they can be replaced collectively when the performance is deemed unsatisfactory. Second, all RGFs are required to be working in order to handle the peak demand. Many water treatment works have multiple RGFs, and the failure of one or more components within the system impacts the ability to handle the peak demand. The loss of this service level is regarded as a system failure.

In this study, we propose the work load adjustment strategy discussed above and exemplified in the context of a water filtration process. The work load adjustment strategy can be generalized to a wider class of infrastructure systems which involves human operators with the ability to make process interventions. Such systems can be regarded as load-sharing systems, since the components share the total work load of the system according to a defined set of load-sharing rules [7–10]. Reliability modeling of a load-sharing system with managed component degradation is challenging because components in the system are used intermittently.

Previously, Ye et al. [11] proposed a framework to model reliability based on the concept of cumulative work load (CWL). As shown in Fig. 1, the framework consists of three steps. The first step focuses on the CWLs-to-failure, initially at component level then the aggregation of component CWL-to-failure to obtain the system CWL-to-failure. The second step obtains the CWL process for the system. The last step combines the system CWL-to-failure distribution and the CWL process to reach the time-to-failure of the system. The merits of this framework are as follows. First, components are used intermittently and thus it is not appropriate to use the chronological time scale. The CWL reflects the component usage, which affects the degree of component degradation. Second, data can be more routinely recorded for the CWL process to the system and the component CWLs-to-failure, meaning that data are likely to be available to populate the model. Third, the model allows us to obtain the system CWL-to-failure from data captured for the component CWLs-to-failure. For example, when a system comprising multiple components is replaced because the managed degradation levels of all components are beyond an acceptable threshold, then we have multiple component CWLs-to-failure. Statistical inference based on the component CWLs-to-failure will be more accurate given the data. Even if empirical records are not available, then subject matter expertise can be elicited from, say, relevant operators who can reason through the probabilistic uncertainties in the CWL-to-failure based on their greater understanding and experience of component, rather than system, failures [12].

Based on the framework in Fig. 1, the model in [11] is able to predict the expected cost of a new system within a given mission time. However, this model has two limitations. First, the model is relevant for new systems only; it is not able to model systems which have accumulated operational experience and partial degradation. The existing model can support decisions taken by a utility when introducing new assets during, for example, new or replacement installations by determining the optimal number of components for the system [11]. However, the model is not able to support decisions regarding

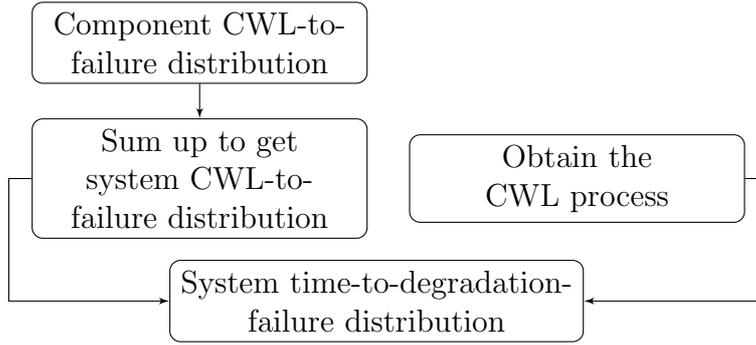


Figure 1: A framework for a load-sharing system with managed component degradation proposed in [11].

the management of assets of varying experience and degrees of degradation. For example, a water utility may have RGFs with different operational experience between water treatment works. Second, the model does not have the capability to represent situations where there are multiple systems, each with one or more components that are operated simultaneously, and a centralized maintenance resource allocation process. For example, consider the geographical dispersion of the sites of water treatment works, i.e. the system, for which we want to model the RGFs as key components. The heterogeneity in site location implies variation in environmental factors which can impact, for example, input water quality and volume. Asset managers tend to believe that for the same volume of water processed, sites which process water with higher levels of impurity will degrade faster. Additionally, each water treatment works will typically have its own operators. The experience and skill of the operators can also vary and so it is reasonable to assume that sites processing the same volume of water with identical levels of impurity may degrade at different rates due to operator effects.

In view of these two deficiencies, this study extends the model of [11] in two distinct ways. First, we develop a cost prediction model for an in-service system. Second, we develop a model to capture the differences in the degradation of components across multiple systems. A random effect model is developed to incorporate the impact of influential factors that contribute to the rate of degradation. For both model developments, statistical inference of the necessary parameters are discussed. The first model development provides expected cost predictions which can aid understanding of future failure profiles and hence resource implications for asset management. The second model development extends the analytical capability to account for the effects of influencing factors so that more meaningful comparisons can be made between systems which are competing for resource.

The contribution of this paper is twofold. Scientifically, the development of the modeling methodology makes a distinctive contribution to the existing literature on reliability models for load-sharing systems. More practically, the models are aligned with asset management having been motivated by and illustrated using industry data from a water utility. Indeed in reality, this research has been conducted to support analytical specialists within the water utility by providing more advanced modeling methods, from which decision support tools for asset managers can be created.

The remainder of this paper is organised as follows. Section 2 presents the key assumptions underpinning the modeling approach. Section 3 develops a cost model for a system with components that have been in operation for a period of time. Section 4 mod-

els the impact of influencing factors in changing the component CWL-to-failure. Section 5 presents inference methods for both a single system and multiple systems. Section 6 illustrates the logical behavior of the model as well as showing the principles of analysis to aid asset managers. Section 7 concludes and shares suggested further work.

2 Model Setting for a Single System

Consider a system with N functionally-identical components. Assume that the components are subject to degradation failures only. Our model is based on the concept of CWL. The motivation for using the CWL concept is that the cumulative usage of a component induces its degradation and CWL can be observable in practical contexts. We assume the failure rate of a component increases in its CWL, and the CWL-to-failure is different for each component. Hence we model the CWL-to-failure as a random variable. While CWL can be considered a measure of cumulative exposure as used in, for example, [13] who described the effect of a dynamic covariate on the failure time distribution, our study models the cumulative work load as a stochastic process, which is based on a physical interpretation of degradation of a system of components. Interested readers may refer to [11] for a detailed illustration of the CWL-to-degradation-failure framework. In general, the following assumptions are used for a single system.

- The CWL to the system is a monotone increasing stochastic process $U(t); t \geq 0$, which has stationary increments. That is, for $t, s > 0$, $U(t+s) - U(s)$ is independent of $U(s)$, has positive support, and has the same distribution as $U(t)$. Here $t = 0$ stands for the date of analysis (say, today), which is different from the installation date of the system.
- During system operation, the operator dynamically allocates the work load to the components based on the work load magnitude and the component degradation levels. Through the work load adjustment, the degradation levels of all components are managed so that the time to degradation failure of each component does not differ too much from each other.
- The component CWL-to-degradation-failures $X_i, i = 1, \dots, N$, are independent and identically distributed (i.i.d.) and follow an inverse Gaussian (IG) distribution, i.e., $X_i \sim \mathcal{IG}(\mu, \lambda)$, $\mu, \lambda \geq 0$, with probability density function (PDF) and cumulative distribution function (CDF) as

$$f_{\mathcal{IG}}(x; \mu, \lambda) = \left(\frac{\lambda}{2\pi x^3} \right)^{\frac{1}{2}} \exp \left(-\frac{\lambda(x - \mu)^2}{2\mu^2 x} \right),$$

and

$$F_{\mathcal{IG}}(x; \mu, \lambda) = \Phi \left(\sqrt{\frac{\lambda}{x}} \left(\frac{x}{\mu} - 1 \right) \right) + \exp \left(\frac{2\lambda}{\mu} \right) \cdot \Phi \left(-\sqrt{\frac{\lambda}{x}} \left(\frac{x}{\mu} + 1 \right) \right),$$

for $x > 0$, where $\Phi(\cdot)$ is the standard normal CDF.

We briefly discuss the justification for the above assumptions. We assume components only suffer degradation failures. This assumption is commonly used for systems or products when the wear/degradation dominates failure, e.g., the contact image sensor of fax

machines [14], the railway tracks [15], and the print head of color printers [16], to name a few. In the industry problem we consider in Section 6, the component subject to failure is the RGFs in a water utility. RGFs are used to filter impurities from raw water. The wear of RGFs occurs during the usage and, as a result, assuming that components are mainly subject to degradation-induced failures is reasonable. Further, we assume a component fails when its cumulative work load exceeds a random threshold and propose that this framework may be more flexible to characterize the failure behavior compared with the models traditionally reported in the literature, where the component fails when its observable degradation level exceeds a fixed failure threshold. Following [11], we assume the IG distribution can provide a reasonable model for the CWL-to-degradation-failure.

Based on the CWL framework, the system fails when the system CWL $U(t)$ exceeds the random threshold $S \equiv \sum_{i=1}^N X_i$, which can be readily shown to follow an IG distribution $S \sim \mathcal{IG}(N\mu, N^2\lambda)$ [11]. This random variable S is the system CWL-to-degradation-failure. On the other side, we can use Gamma process, IG process or compound Poisson process to model the system CWL process $U(t)$. If the variation of the CWL process is small, we may use a linear function to approximate the CWL process. As an example, if the CWL process follows a Gamma process with scale parameter γ and shape parameter v , then the PDF of the system CWL at time t is given by

$$g_{U(t)}(u; vt, \gamma) = \frac{u^{vt-1}}{\Gamma(vt)\gamma^{vt}} \exp\left(-\frac{u}{\gamma}\right), \quad u > 0. \quad (1)$$

3 Cost Analysis of an In-Service System

For an in-service system, we treat the replacement process as a delayed renewal process. This section derives the expected cost for such a system that has been operational for a period of time at $t = 0$. An algorithm is proposed to compute the renewal function. The rate of convergence of the numerical error is shown to be $O(n^{-1})$.

3.1 The replacement process – a delayed renewal process

At time $t = 0$, the (N -component) system of interest may have been in operation for a period of time. For this system, the replacement process constitutes a delayed renewal process $\{Y, T_1, T_2, \dots\}$ with the i.i.d. inter-replacement times $T_i, i = 1, 2, \dots$, and the first replacement time Y following a different distribution. Here, Y can be regarded as the remaining useful life of the system. The distribution of Y depends on how long the system has been in use and how much work load it has processed. If we only know that the system has been in operation for z units of time at $t = 0$, the distribution of Y is given by

$$F_Y(t) = \Pr(T > t + z | T > z) = \frac{F_T(t + z) - F_T(z)}{1 - F_T(z)}.$$

If in addition to the operation time z , the CWL of the system at time $t = 0$ is available, then we can have a more accurate estimation for Y because the lifetime of the system is determined by its usage. Denote the CWL at $t = 0$ as u and thus the CWL at time t is $U(t) + u$. Given u at time 0, the distribution of Y can be computed as

$$F_Y(t) = \Pr(S < U(t) + u | S > u) = \frac{\Pr(u < S < U(t) + u)}{\Pr(S > u)}.$$

The numerator can be expressed as $\Pr(u < S < U(t)+u) = \Pr(S < U(t)+u) - \Pr(S < u)$, where $\Pr(S < u)$ is simply the CDF of the IG random variable $\mathcal{IG}(N\mu, N^2\lambda)$, while $\Pr(S < U(t) + u)$ can be evaluated as

$$\Pr(S < U(t) + u) = \int_0^\infty F_S(v + u)g_{U(t)}(v)dv. \quad (2)$$

3.2 Expected cost over a finite time horizon

Based on the above assumption, the replacement consists of a delayed renewal process $\{Y, T_1, T_2, \dots\}$, where the system starts anew after each replacement. Let $N(t)$ be the number of system replacements at time t , and $m^*(t) = E[N(t)]$ is the renewal function for the delayed renewal process. Suppose that the unit time operation cost of an N -component system is NC_O , depending on the number of components. When the system is replaced because of unsatisfactory performance, the replacement cost includes the price of each component, denoted by C_r , and a fixed system replacement cost, denoted by C_s . All the components are replaced at the same time for the two reasons as discussed in Section 1. The simultaneous replacement can be achieved through the work load adjustment by operators. In our case study in Section 6, the RGFs degrade/wear gradually with usage. The degradation level of the filtering media cannot be measured precisely. Nevertheless, an operator can assess the degradation level by directly observing the filtering media. For example, dark spots and darkening of the media on visual inspection indicate that the RGF is aging. The measurements of output water quality from the RGFs can also help to assess the degradation level. Operators then select which RGF(s) to use based on their observation of the RGF degradation levels. Under this strategy, the degradation levels of all RGFs are balanced, so that all RGFs in a system fail approximately at the same time. The replacement time is assumed negligible compared with the time-to-degradation-failure of the system. The expected cost of an existing system operating within a predetermined time horizon τ can be expressed as

$$C(\tau) = N\tau C_O + (NC_r + C_s)m^*(\tau). \quad (3)$$

The expected cost (3) involves the delayed renewal function, which has to be evaluated numerically. Here we develop a numerical procedure to compute the delayed renewal function. The procedure is based on the Riemann-Stieltjes (RS) sums method by partitioning $[0, \tau]$ into n sub-intervals.

We now consider the following delayed renewal process $\{Y, Z_1, Z_2, \dots, Z_n, \dots\}$ where Y has a survival function (SF) $\bar{F}^*(t)$ and $Z_n, n \geq 1$ have a SF $\bar{F}(t)$. Conditional on the first arrival, $m^*(t)$ can be expressed as

$$m^*(t) = F^*(t) + \int_0^t F^*(t-x)dm(x), \quad (4)$$

where $m(t)$ is the renewal function for the renewal process $\{Z_1, Z_2, \dots, Z_n, \dots\}$ with renewal equation

$$m(t) = F(t) + \int_0^t F(t-x)dm(x). \quad (5)$$

Xie [17] proposed a simple numerical method to compute $m(t)$ based on the RS sums. To use this method, the interval $[0, \tau]$ is uniformly divided with $0 = t_0 < t_1 < \dots < t_n = \tau$.

Then $m(t_j), 1 \leq j \leq n$, can be numerically calculated iteratively. Its expression was derived by [17] and is given by

$$\tilde{m}(t_j) = \frac{F(t_j) + G_j - F(t_j - t_{j-1/2})\tilde{m}(t_{j-1})}{1 - F(t_j - t_{j-1/2})}, \quad (6)$$

where $\tilde{m}(t_0) = 0$, $t_{j-1/2} = (t_{j-1} + t_j)/2$ and $G_j = \sum_{k=1}^{j-1} F(t_j - t_{k-1/2})[\tilde{m}(t_k) - \tilde{m}(t_{k-1})]$. We use $\tilde{m}(t_i)$ to indicate that it is a numerical approximation of $m(t_i)$. Once $\tilde{m}(t_1), \tilde{m}(t_2), \dots, \tilde{m}(t_j)$ are calculated, we can approximate $m^*(t_j), 1 \leq j \leq n$ based on the RS sums method, which is given by

$$\tilde{m}^*(t_j) = F^*(t_j) + \sum_{k=1}^j F^*(t_j - t_{k-1/2})[\tilde{m}(t_k) - \tilde{m}(t_{k-1})].$$

According to our numerical experience, the above numerical method for $m^*(t_n)$ is very accurate. The following theorem shows that the approximation error of the numerical procedure is $O(n^{-1})$. The proof is given in the appendix.

Theorem 1. *Suppose $F(\cdot)$ is twice differentiable in the interval $[0, \tau]$ with first derivative f and bounded second derivative, and $m(\cdot)$ and $F^*(\cdot)$ are differentiable in $[0, \tau]$ with bounded derivatives. If $\max\{t_i - t_{i-1}; i = 1, 2, \dots, n\} = O(n^{-1})$, then $m^*(t_n) - \tilde{m}^*(t_n) = O(n^{-1})$.*

4 Heterogeneities of Multiple Systems

In situations where multiple systems of the same type exist, there can be system-to-system differences in their degradation profiles. For example, a water utility has multiple water treatment sites and the raw water quality in one water treatment site may be significantly different from another site in a different location. Given the same functional performance of the filtering media, the CWL process depends on the quality of the raw water. It is expected that the more impurities in the raw water, the faster the degradation rate of the media and thus the shorter the CWL until failure. This implies that the quality of raw water affects the CWL-to-failure of a filter. Other possible causes of the heterogeneities include operators and environments. They are called influential factors in our study.

If information about the influential factors is available, then we can treat them as a covariate and incorporate into the component CWL-to-failure distribution. We assume that the influential factor is constant at a site. For example, data is available on incoming raw water quality to each water treatment site, hence we can model this factor as a constant y in a site, where a higher y means a better quality of the raw water, i.e., slower degradation of the RGF. Recall that the component CWL-to-failure $X \sim \mathcal{IG}(\mu, \lambda)$. The convention to model covariates is to let some parameters in the distribution of X be an increasing function of y . A larger y leads to a larger CWL-to-failure X in general, while $E[X] = \mu$. Therefore, we can assume μ to be a function of y . Some common choices for the link function between y and μ are the power law, the Arrhenius law and the exponential law [18, 19]. An appropriate link function can be determined based on expert knowledge, or from data analysis. Suppose that we have chosen the power law, i.e., $\mu = ay^b$, where $a, b > 0$ are parameters. Then the system CWL-to-failure $S \sim \mathcal{IG}(Nay^b, N^2\lambda)$.

If we do not have enough information on the impact of the influential factors (e.g., data have not been collected for each site), then the impact of the covariate becomes an

unobserved factor that affects the CWL-to-failure of a filter. Unobserved factors affecting the CWL-to-failure of a RGF include the skills of operators and local site environments. For example, high operator skill affects key process steps such as the quality of the backwash, which is a regular procedure to cleanse the media. If the media is less clean, then the RGF needs to work harder and therefore degrades faster. Systematic difference in the CWL process between water treatment sites can occur because different locations serve areas with different population size and thus different demand rates. Local environments, e.g., humidity and temperature, also affect the demand rate and the CWL process.

The unobservable factors in the CWL process and the CWL-to-failure of the components (e.g., the RGFs) can be captured by random effects. Essentially, the random effects are unobserved covariates which vary over sites. The random effect in each unit is fixed yet unknown (e.g., the impurities of the raw water), and different units have different realizations of the random effects [20–24]. A special case of random effects is the frailty, which assumes the unobserved covariates have a multiplicative effect on the baseline failure rate [25, 26]. For a comprehensive overview of frailty models, see, for example, [27]. In the following, we discuss how to use random effect model to capture these unobserved factors.

First of all, we discuss incorporation of random effects in the lifetime distribution for the component CWL-to-failure. For a system, we have assumed that the component CWL-to-failure $X \sim \mathcal{IG}(\mu, \lambda)$ while the system CWL-to-failure $S \sim \mathcal{IG}(N\mu, N^2\lambda)$. The environmental factors, (e.g., the raw water impurity) influence the means of X and S . Therefore, we can assume that μ is fixed and unknown for each site, and is random across all sites. The distribution of μ can be obtained from data analysis, as discussed in Section 5, or from expert knowledge [28]. One convenient method is to use the conjugate prior, which assumes $1/\mu \sim \mathcal{N}(v, \sigma^2)$. To derive the component CDF of the system CWL-to-failure in this case, we need the following lemma.

Lemma 1. *Suppose $Z \sim \mathcal{N}(v, \sigma^2)$. Then we have*

$$E[\Phi(aZ + b)] = \Phi\left(\frac{av + b}{\sqrt{1 + a^2\sigma^2}}\right) \quad (7)$$

and

$$E[e^{cZ+d}\Phi(aZ + b)] = \exp\left(cv + d + \frac{1}{2}c^2\sigma^2\right) \times \Phi\left(\frac{av + b + ac\sigma^2}{\sqrt{1 + a^2\sigma^2}}\right), \quad (8)$$

where a, b, c and d are arbitrary constants.

The proof of Lemma 1 is given in the appendix. With the lemma, we can obtain the component CDF of the system CWL-to-failure as

$$F_X(x) = \Phi\left(\frac{vx - 1}{\sqrt{x/\lambda + \lambda\sigma^2}}\right) + \exp(2\lambda + 2\lambda^2\sigma^2) \Phi\left(\frac{-(v + 2\lambda\sigma^2)x - 1}{\sqrt{x/\lambda + \lambda\sigma^2}}\right). \quad (9)$$

The detailed procedure to derive Equation (9) is also given in the appendix. It should be noted that conditional on the random effects μ , the CWLs-to-failure of components in the same system are independent. However, they are dependent unconditionally. The CDF of system CWL-to-failure can be computed by integrating μ out of $\mathcal{IG}(N\mu, N^2\lambda)$, which yields

$$F_S(s) = \Phi\left(\frac{vx - N}{\sqrt{x/\lambda + N^2\lambda\sigma^2}}\right) + \exp(2N^2\lambda + 2N^2\lambda^2\sigma^2) \Phi\left(\frac{-(v + 2N\lambda\sigma^2)x - N}{\sqrt{x/\lambda + N^2\lambda\sigma^2}}\right).$$

Next, we explain how we might capture random effects on the work load accumulation process for the cases of the Gamma process. We may assume that the CWL to system follows the Gamma process $U(t) \sim \text{gamma}(vt, \gamma)$ with the PDF given in (1). Note that the mean of $U(t)$ is γvt . We have argued that different sites will have different demand rate and so different accumulation rates for the CWL. Therefore, we may assume that γ is different across sites. A tractable model results when γ^{-1} follows a Gamma distribution with $\gamma^{-1} \sim \text{gamma}(\kappa, \phi^{-1})$. Integrating γ out of Equation (1) yields the unconditional distribution of $U(t)$ as

$$g_{U(t)}(u) = \frac{\phi^\kappa \Gamma(vt + \kappa) u^{vt-1}}{\Gamma(\kappa) \Gamma(vt) (\phi + u)^{vt+\kappa}}.$$

5 Statistical Inference

5.1 Single system

First we return to the scenario where we consider only one system (e.g., a water treatment site) which implies the random-effects model is not necessary. Under this scenario, we need to estimate the CWL-to-failure distribution and the CWL process. The CWL-to-failure distribution can be estimated using the usage data of each component upon replacement. Such usage data are available, e.g., from a meter embedded in the component. Let $\{x_1, x_2, \dots, x_N\}$ be the CWL-to failure of N components, e.g., from several replacements. Inference for the IG distribution has been well-addressed in the literature, e.g., Section 11.4 in [29]. We briefly recapitulate the procedure here. The log-likelihood function, up to a constant, is

$$l(\mu, \lambda) = \frac{1}{2} N \ln \lambda - \sum_{i=1}^N \frac{\lambda(x_i - \mu)^2}{2\mu^2 x_i}. \quad (10)$$

Maximizing this function yields the maximum likelihood estimators (MLEs) of μ and λ as $\hat{\mu} = \bar{x}$, the average of x_i , and $\hat{\lambda} = N / \sum_{i=1}^N (x_i^{-1} - \bar{x}^{-1})$.

To estimate the CWL process, suppose we collect the CWL periodically, say, every day or every week. Let $\mathbf{U} = \{u_1, u_2, \dots, u_K\}$ be the consecutive CWL measurements from period 1 to period K and $\Delta \mathbf{u} = \{\Delta u_1, \Delta u_2, \dots, \Delta u_K\}$ be the increments, i.e., $\Delta u_i = u_i - u_{i-1}$ with $u_0 = 0$ for convenience. Because we have assumed that the process has stationary increments and the data collection interval is constant, the increments Δu_i should be i.i.d. samples. If we assume the CWL accumulation is a Gamma process, then we can use a Gamma distribution to fit $\Delta \mathbf{u}$. Let Δt be the collection time interval. Then the increments follow $\text{Gamma}(v\Delta t, \gamma)$ and thus the log-likelihood function, up to a constant, is given by

$$l(v, \gamma) = K[v\Delta t \ln \gamma - \ln \Gamma(v\Delta t)] + v\Delta t \sum_{i=1}^K \ln \Delta u_i - \gamma \sum_{i=1}^K \Delta u_i.$$

The MLE can be obtained by first solving $\ln(v\Delta t/\bar{u}) + \overline{\ln u} = \psi(v\Delta t)$ for \hat{v} , where $\psi(\cdot)$ is the digamma function, and \bar{u} and $\overline{\ln u}$ are averages of u_i and $\ln u_i$, respectively. Then $\hat{\gamma}$ is given by $\hat{\gamma} = \hat{v}\Delta t/\bar{u}$.

5.2 Multiple systems

Now consider the situation where we model multiple sites. If the raw water quality for each site is available then we can treat this factor z as a covariate and assume that μ is a function of y . Suppose that we choose to use the power law relation $\mu = ay^b$, and that data on M sites are available. For the i th site, the CWL-to-failure data are $\{x_{i1}, x_{i2}, \dots, x_{iN_i}\}$ and the raw water quality level is y_i ; and suppose the CWL process is observed at time epochs $\{t_{i1}, t_{i2}, \dots, t_{iK_i}\}$ with the associated CWL $\mathbf{u}_i = \{u_{i1}, u_{i2}, \dots, u_{iK_i}\}$. Then the log-likelihood function is a slight modification of (10):

$$l(\mu, \lambda) = \sum_{i=1}^M \left[\frac{1}{2} N_i \ln \lambda - \sum_{j=1}^{N_i} \frac{\lambda(x_{ij} - ay_i^b)^2}{2(a^2 y_i^{2b}) x_{ij}} \right]. \quad (11)$$

If the raw water quality is not observed, we use the random-effects models to capture the differences in the component CWL-to-failure distribution between different sites. Denote $\omega_{i1} = \sum_{j=1}^{N_i} x_{ij}$, $\omega_{i2} = \sum_{j=1}^{N_i} x_{ij}^{-1}$ and $\omega_{i3} = \sum_{j=1}^{N_i} \ln x_{ij}$. We then use the following lemma to derive the log-likelihood function.

Lemma 2. *Let $Z \sim \mathcal{N}(v, \sigma^2)$. Then we have*

$$E \left[\exp \left(-\frac{1}{2} (aZ^2 + 2bZ + c) \right) \right] = \begin{cases} (1 + a\sigma^2)^{-\frac{1}{2}} \exp \left[-\frac{2bv + av^2 - b^2\sigma^2}{2(1+a\sigma^2)} - \frac{1}{2}c \right], & a > -\sigma^{-2}; \\ \infty, & a < -\sigma^{-2}. \end{cases}$$

Proof of the lemma is straightforward and thus it is omitted here. The log-likelihood function, up to a constant, can be then expressed as

$$l(\lambda, v, \sigma^2) = \frac{1}{2} \sum_{i=1}^M \left\{ N_i \ln \lambda - \ln(1 + \lambda\sigma^2\omega_{i1}) - \frac{\lambda\omega_{i1}v^2 - 2N_i\lambda v - N_i^2\lambda^2\sigma^2}{1 + \lambda\omega_{i1}\sigma^2} - (\lambda\omega_{i2} + 3\omega_{i3}) \right\}. \quad (12)$$

Detailed derivation of (12) is given in the appendix. Direct maximization of this function yields the MLEs of the three parameters.

We now discuss statistical inference of the CWL process given that there are random-effects between different sites. Since we assume that the system CWL process follows the Gamma process with random effect, the likelihood function is given by

$$l(v, \kappa, \phi) = \sum_{i=1}^M \kappa \ln \phi + \ln \Gamma(vt_{iN_i} + \kappa) - \ln \Gamma(\kappa) - (vt_{iN_i} + \kappa) \ln(\phi + u_{iN_i}) + \sum_{i=1}^M \sum_{j=1}^{N_i} v \Delta t_{ij} \ln \Delta u_{ij} - \ln \Gamma(v \Delta t_{ij}). \quad (13)$$

Since there are only three parameters, most optimization software packages can maximize this function efficiently.

6 Example to Examine Model Behavior and Use

We use de-sensitized real data from selected water treatment sites owned by a water utility to illustrate the application of our proposed models. Tables 1 - 3 in the appendix provide the data for the CWL-to-failure of RGFs at fifteen water treatment sites, together with the typical CWL per annum and the average water turbidity at each of the sites. Turbidity is a measure of water quality and one intent of filtering is to reduce turbidity. While turbidity will naturally vary throughout the year for each site, we have taken a yearly average which is requisite for our example purpose since this average provides an indicative level of the typical raw water impurity. Using monthly demand from each site over an annual period, data which is recorded by the water utility although not shown, we can estimate the CWL process. The raw water quality can then be treated as a covariate. Note also that each site contains a different number of RGFs and, although not shown, it is known that each site is of a different size, has different operators, has different work load profiles and has filters of different ages.

Currently, the water utility forecasts the operation costs of their water treatment sites within a 5-year time horizon, so that they can assess the variation of maintenance and replacement spending from year to year for the multiple treatment sites. Motivated by this, we propose a cost prediction model with the consideration of the system heterogeneity and the system current usage condition. Section 6.1 explains how we estimate model parameters from the data. Section 6.2 uses these estimates to compute the failure time distribution, the expected failure number, and the expected operational cost for a single system, and discusses the results for site 1 with the main purpose of sense-checking the model behavior. Section 6.3 then predicts the operational costs for multiple sites and discusses how the findings allow us to anticipate when the peaks in the cost spending may occur across sub-sets of the sites, and to illustrate how asset managers might gain insight into anticipated cost and failure profiles. Section 6.4 presents some sensitivity analysis and discusses the implications for our understanding of the model and its results.

6.1 Estimation of model parameters

We use an IG distribution with mean parameter $\mu = ay^b$ and shape λ for the CWL-to-failure of the RGF components, where y is the turbidity for the site. Based on the data, the estimates of a , b and λ are $\hat{a} = 991$, $\hat{b} = -1.50$ and $\hat{\lambda} = 14285$. These estimates are obtained by numerically maximizing the log-likelihood function (11), which can be achieved by most optimization functions in modern software, e.g., `fminsearch` in MATLAB.

We then use the monthly demand data to estimate the CWL process by assuming a Gamma process with random effects. By setting the time unit as one month, the estimates of parameters ϕ , κ and v are $\hat{\phi} = 98.15$, $\hat{\kappa} = 3.138$ and $\hat{v} = 1.038$ through numerically maximizing the log-likelihood function (13). Let $t_{\max} = 12$ and $U_i(t_{\max})$ be the CWL of the i th system at the end of the year. Then γ_i^{-1} for the i th system follows a Gamma distribution with shape parameter $\hat{v}t_{\max} + \hat{\kappa}$ and scale parameter $(U_i(t_{\max}) + \hat{\phi})^{-1}$. We can compute the estimates of γ for the 15 sites as

$$\hat{\gamma} = [118, 63.0, 119, 24.3, 36.2, 37.0, 30.4, 59.4, 39.5, 37.8, 35.9, 17.5, 23.8, 21.8, 21.7].$$

Based on these results, we can insert the parameter estimates into the model and compute the expected cost. We assume that the operational cost for each component is $C_O = 0.05$ per year, the price of a component is $C_r = 1$ and the fixed replacement

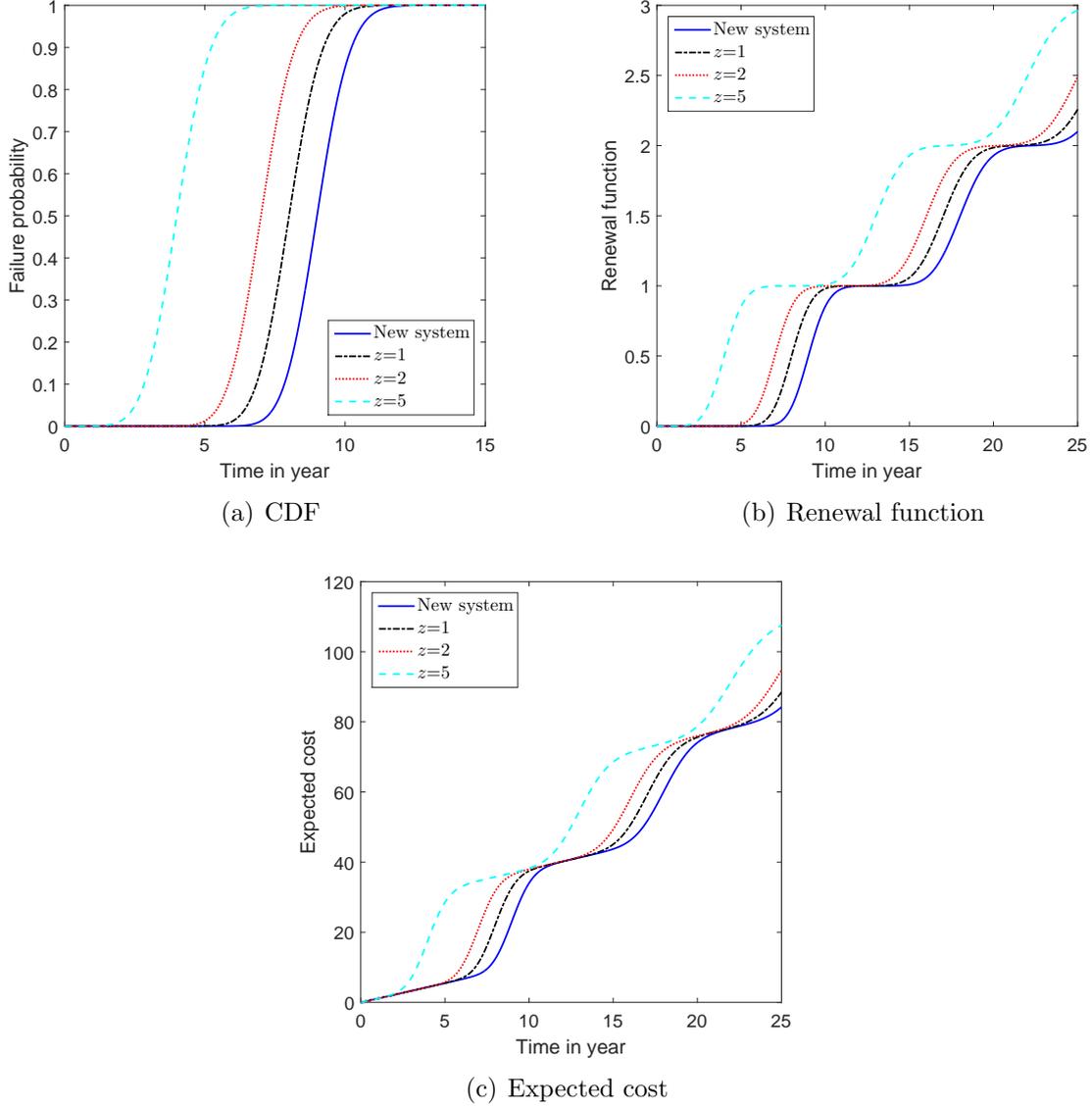


Figure 2: The CDF, renewal function and expected cost for Site 1 with specified years of past operation only.

cost is $C_s = 5$. The units used for the time, monetary values and the CWL are in years, thousands of pounds and trillion of litres, respectively.

6.2 Failure behavior for a single site

Consider site 1 to illustrate the behavior of the model. For site 1, the average turbidity value is $y = 1.4$. From this, $\hat{\mu} = \hat{a}y^b = 598.2$. First, let us consider the situation where the system has been in operation for a period of time prior to analysis. Suppose we know that the system has been in operation for z years at time $t = 0$ and let us consider the cases where $z = 1, 2$ and 5 years. The CDFs for the time to first replacement of the system are shown in Fig. 2(a), while the renewal functions and the expected costs over time are depicted in Fig. 2(b) and 2(c). The CDF of a new site 1 is nearly zero at the first five years and it then increases rapidly during the time from 5 to 10 (year). Meanwhile, the failure probability after 10 years is nearly one. Since the renewal function equals the

expected number of failure during a time horizon, the renewal function at the first five years should be near zero, as shown in Fig. 2(b). Similarly, the renewal function at time 10 (year) should be approximately equal one since there is a very high probability that the system has failed before this time. The system is replaced with a new one after failure, so there should be no failures at the first five years after the replacement. This explains why the renewal function keeps nearly constant from 10 to 15 (year). From Equation (3), the expected cost $C(\tau)$ is a function of the renewal function $m^*(\tau)$. When $m^*(\tau)$ keeps nearly constant, the increase of $C(\tau)$ is mainly due to the operational cost. Because the operational cost is much smaller than the replacement cost for a system, the increase of $C(\tau)$ is relatively slow during [10, 15] and [20, 25] (year) for the new site, as shown in Fig. 2(c). We can also see that the CDF shifts to the left as z increases as expected, since older systems with longer operational experience will have accrued greater degradation.

6.3 Analysis using the model for multiple sites

Now consider how the model can be developed for analytical support. Typically, there are constrained budgets for portfolios of multiple assets. The proposed model can aid managers to anticipate when peaks in spending might occur by providing insight about the failure profiles of individual assets and the associated maintenance costs.

Using the same data and model as the previous section, Fig. 3 shows the degradation failure profiles for the RGFs at the sites of five water treatment works over a 25-year horizon. The sites vary in terms of number of RGFs as well as the age (z) and CWL (u) at the time of analysis, $t = 0$. The predictions show that sites 1 and 3 are almost certain to fail within the next 5 years, while the failure profiles of sites 11 and 15, in particular, are shifted to the right with only a 0.5 chance of failing in around 15 and 19 years respectively. The two sites whose CDFs indicate a higher median time to failure and greater variation are smaller in terms of the number of RGFs installed, younger in terms of calendar age as well as CWL, and have low turbidity in comparison with the three sites whose CDFs indicate failures are more likely to occur within a relatively shorter time scale. Sites 1 and 3 are very similar in all respects except the number of RGFs. Site 3, which has half the number of RGFs but a similar high average turbidity level as site 1, has the most left failure profile. For site 3, there is a 0.5 chance of failure in about 2.5 years from the time of analysis compared with 4.5 years for site 1. Site 13 has fewer RGFs but a mid-level average turbidity suggestive of a relatively high work load, and the model predicts a 0.5 chance of failure within around 4 years. While this prediction indicates failure may be earlier than for the largest site 1, there is greater variation in the CDFs for site 13 compared with sites 1 and 3. For example, there is a 0.9 chance that site 13 will fail in about 6 years but that site 1 will fail within 5 years.

Fig. 4 shows the expected cost predicted for a selection of sites, which are those presented in Fig. 3 plus site 8. The cost profiles are based on the delayed renewal model and it is assumed the fixed replacement cost, the annual operational cost of failure and the price of an RGF are the same for all sites. This assumption can be readily relaxed if financial values of different sites are all available. The expected costs profiles show how the CDFs translate to the monetary values for sites of different sizes. Consider sites 1 and 3. The increased rate of failure over the 3 years or so from the point of analysis for site 3 corresponds to the greater chance of failure over this planning horizon compared with site 1. The lesser number of RGFs in site 3 compared with site 1 means that the expected cost profile is lower and that curvature, indicative of replacement to renew the

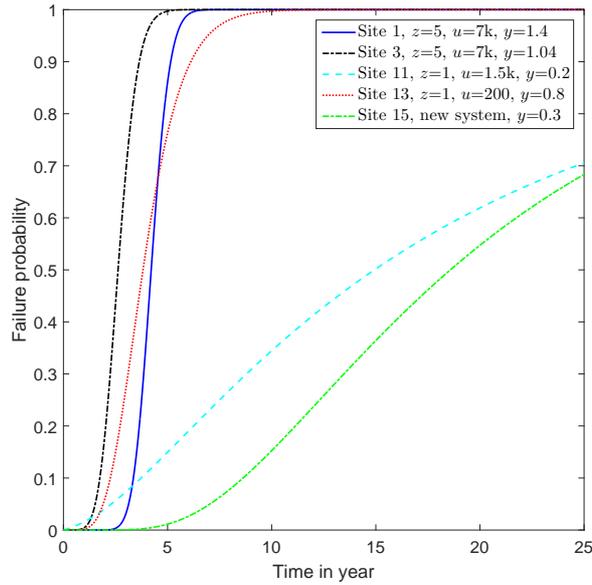


Figure 3: CDF for selected sites over a 25-year planning horizon.

site RGFs, is more damped. The expected cost profiles of sites 11, 13 and 15 are lower as might be anticipated given their CDF profiles and the single RGF. Site 8 with three RGFs and two years of past operation has a cost profile higher than those for sites 11, 13 and 15, but it is still relatively much lower than the costs predicted for sites 1 and 3. Fig. 4 shows that it is expected £30m and £19m will be the costs associated with the failure and replacement of sites 1 and 3, respectively. Given a specified planning horizon, the asset manager can accumulate expected costs across sites within his portfolio.

Fig. 5 shows both the CDF and expected cost profiles for four sites, all with average turbidity levels above the median for the set of 15 sites analyzed. Hence we focus upon sites where the raw water quality input to the RGFs is relatively less good. Consider the pairing of sites 1 and 2. They differ most in terms of the annual work load which for site 1 is double that of site 2, even though site 2 has the slightly higher average turbidity and one less RGF. Thus for reasons that are not evident in the data provided for analysis, the RGF at each of these sites are being operated in such a way that they have different work loads. Given our earlier discussions, we can hypothesize that this could be due to the existence of process activities (e.g., flocculation) prior to the RGF water filtering stage, operator and /or other effects. The degree of work load appears to affect the comparative location of the predicted failure profiles with the CDF of site 1 positioned to the left of that of site 2. However, despite having a better failure profile, site 2 has a consistently lower expected cost profile than site 1 with greater divergence as the planning horizon gets longer. Similarly, consider the pairing of sites 5 and 12 which both have the same average annual turbidity level, and whose turbidity is just above the median for the 15 sites. However, the relative work load of the RGFs at site 12 is more than four times that of site 5. Even though the size of these sites is very different, the expected cost of site 12 is higher than that of site 5 from year 5 onwards. This might plausibly be due to the dominance of the higher predicted rate of degradation failure in the longer term costing. Fig. 5 provides an illustration of how comparative analysis might be conducted to examine the relative impact of factors over different planning horizons. Such analysis

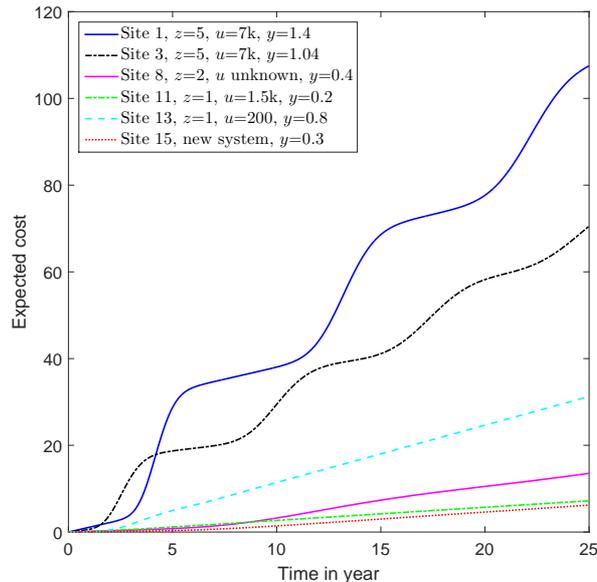


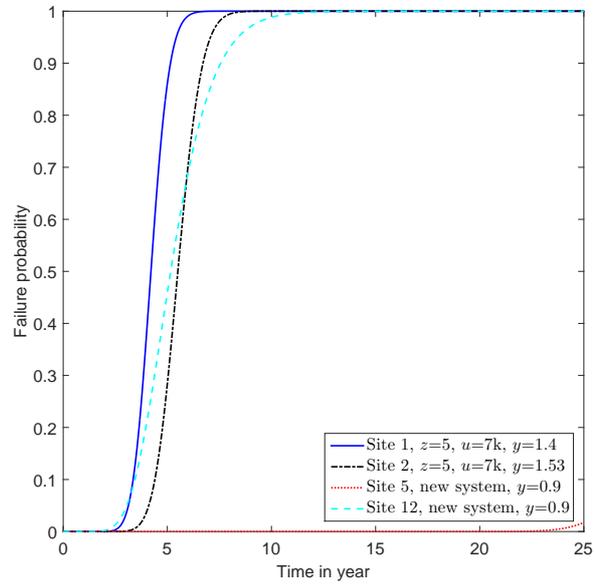
Figure 4: Expected cost for selected sites over a 25-year planning horizon.

allows an initial exploration of site specific effects to make an initial assessment of the likely impact of changing work load through, for example, changes to the operational processes.

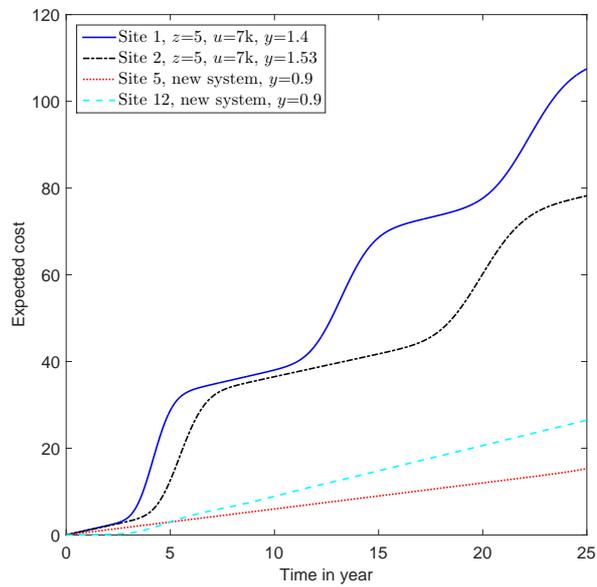
Our analysis shows that the dynamic work load adjustment strategy leads to the variation of the expected annual cost for each site, e.g., see Fig. 4. Since each site has its own degradation behavior, the failure time distribution differs among the treatment sites. As a result, different sites may have different peak years for maintenance and replacement spending. As intended, the proposed model can predict the total cost for each site within a given time horizon and provide asset managers with insight when peaks in the operational cost may occur for each site.

6.4 Sensitivity analysis

To examine the sensitivity of $C(\tau)$ within the time horizon $\tau \in [0, 25]$, we fix the replacement cost, i.e., set $C_r = 1$ and $C_s = 5$ as above, but we change the unit operational cost for one component by setting C_o equal to 0.01 through 0.25, in steps of 0.01. The expected cost $C(\tau)$ is computed for the selected sites in Fig. 3 over a 25-year planning horizon. The results for the two extreme scenario settings are shown in Figure 6. Recall that the operational cost during a time horizon τ is $N\tau C_o$, where N is the number of components in the site. Therefore, the variation of C_o should have the most impact on the sites with a larger N , which are sites 1 and 3 from Table 1. When the operational cost setting is highest, $C_o = 0.25$, then there is little variation of $C(\tau)$ from year to year for sites 1 and 3 and we find that the total cost is around twice as large as that for the base case where $C_o = 0.05$. On the other hand, the total cost $C(\tau)$ is around 75% for sites 1 and 3 when $C_o = 0.01$, which is 20% of the one for the base case. Most strikingly, we find that examination of all sensitivity results, although not shown, indicates that, as the operational cost increases, then the expected cost functions become almost linear. This phenomenon is visually illustrated in Figure 6 for the extreme cases and shows the



(a) CDF



(b) Expected cost

Figure 5: The CDF and expected cost functions for sites 1, 2, 5 and 12.

impact of operating cost increases in dampening the effect of capital replacement costs.

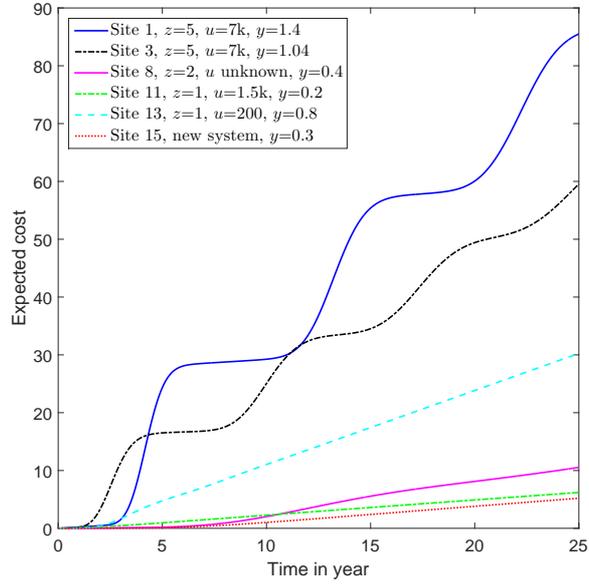
If we focus only upon smaller changes in the unit operational cost for one component, say $C_O = 0.03$ to $C_O = 0.07$ for the base setting of $C_O = 0.05$ within the full set of sensitivity analysis, although all figures are not shown here, then we find that although the absolute values of the expected costs change, their relative profiles do not. Especially in the next 5-year time horizon, we find that the implications for managing assets across the six sites remain the same. Hence within this time window, the results are insensitive to small changes in the operational cost values.

7 Conclusions

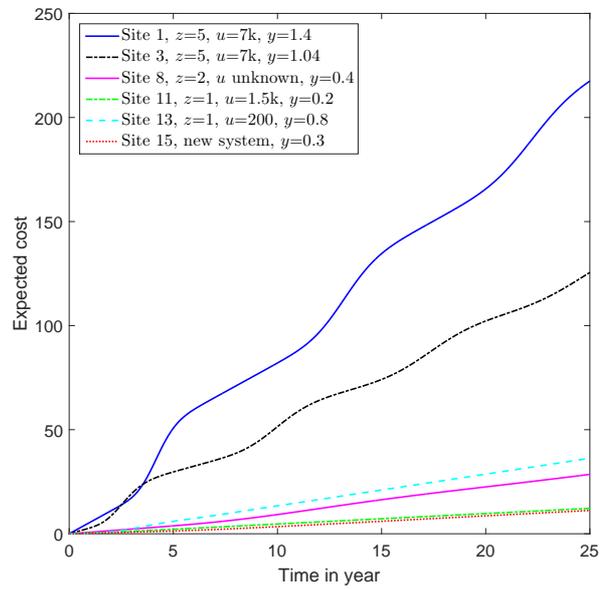
This article has developed a cost prediction model for a load-sharing system which has already accrued operational experience and so is a useful extension of the model originally reported in [11]. Our model is developed for a generalization of a problem motivated by the context of a real infrastructure system, specifically for the water treatment process. We believe that the problem definition is also relevant for other infrastructure contexts, such as wind farms and other power plants. Although the particularities of specific contexts will, of course, impact some modeling choices. For example, the nature of key influential factors and their treatment relevant to data availability, as well as the selection of the underlying probability model. The characteristics of the problem - multiple assets, of different ages, located at different sites, different environmental conditions and different rates of degradation - challenge asset management planning. The modeling methodology developed in this article can be developed to support asset managers address such challenges in the following ways. First, by providing estimates of the expected time of failure and cost of maintenance, the model can provide asset managers with forecasts of spend over defined budget planning horizons. Second, the model has the potential to allow managers to explore the likely impact of investments in other parts of the process to change the workload on key assets and hence impact their degradation failure profile. Finally, the model could be developed to allocate budget.

Scientifically, this article reports method developments associated with the new model. A numerical procedure based on the Riemann-Stieltjes sums has been developed to evaluate the expected cost. The approximation error is shown to be of order $O(n^{-1})$. The model is also able to capture the impact of influential factors on different assets within a portfolio. When these influential factors are observable, we treated them as covariates and incorporated into the CWL-to-failure distribution. When they are unobservable, we treated them as random effects. Statistical inference on the necessary parameters has been developed for both these model variants.

We have used the Gamma process to model the CWL process of the system in this study. We may also develop methods based on other monotone stochastic processes, such as like IG process and compound Poisson process, for fitting the data of CWL process. To deal with the heterogeneity associated with the systems of interest, the random effect should be incorporated for these models. To enhance usefulness, appropriate model selection criteria could be developed to select the best model to fit the data.



(a) $C_O = 0.01$



(b) $C_O = 0.25$

Figure 6: Sensitivity analysis of the expected cost for selected sites over a 25 year horizon to the operational cost C_O .

A Technical Details

A.1 Proof of Theorem 1

For simplicity, suppose $t_i, i = 0, 1, \dots, n$, are equally spaced and let $h = t_i - t_{i-1} = \tau/n$. Abbreviate $F^*(t_i), F(t_i), f(t_i), m(t_i)$ and $m(t_i) - m(t_{i-1})$ as F_i^*, F_i, f_i, m_i and Δm_i .

When x is in the interval (t_{i-1}, t_i) , apply Taylor's expansion to $F(t_k - x)$ at $x = t_{i-1/2}$ to get

$$F(t_k - x) = F_{k-i+1/2} - f_{k-i+1/2}(x - t_{i-1/2}) + O(h^2),$$

where $k \geq i$. Because f has bounded derivative, we can find a common M_1 such that the remainder term is no larger than $M_1 h^2$ for all i . Therefore, we have

$$\int_{t_{i-1}}^{t_i} F(t_k - x) dm(x) = F_{k-i+1/2} \Delta m_i - f_{k-i+1/2} I_i + O(h^3),$$

where $I_i = \int_{t_{i-1}}^{t_i} (x - t_{i-1/2}) dm(x)$. Similarly, because m has bounded derivative, we can find a common M_2 such that the remainder is no larger than $M_2 h^3$, regardless of i . In addition, $\Delta m_i = O(h)$ and thus

$$|I_i| \leq \int_{t_{i-1}}^{t_i} \frac{h}{2} dm(x) = \Delta m_i h/2 = O(h^2),$$

where we can again find a common M_3 such that the above remainder term is bounded by $M_3 h^2$. Therefore, the renewal function m_k given by (5) can be evaluated as

$$m_k = F_k + \sum_{i=1}^k [F_{k-i+1/2} \Delta m_i - f_{k-i+1/2} I_i + O(h^3)]. \quad (14)$$

Apply the above result to m_1 to see

$$m_1 = \frac{F_1 - f_{1/2} I_1 + O(h^3)}{1 - F_{1/2}}.$$

Comparing it with (6), it is obvious that $m_1 - \tilde{m}_1 = O(h^2)$, and $\Delta m_1 - \Delta \tilde{m}_1 = O(h^2)$. Suppose that for $2 \leq i < j$, we have $m_i - \tilde{m}_i = O(h)$, $\Delta m_i - \Delta \tilde{m}_i = O(h^2)$. Based on (14), we have Equation (15) on the top of the next page.

$$m_j = \frac{F_j + \sum_{k=1}^{j-1} F_{j-k+1/2} \Delta m_k - F_{1/2} m_{j-1} + \sum_{i=1}^j [-f_{j-i+1/2} I_i + O(h^3)]}{1 - F_{1/2}} \quad (15)$$

$$m_j - \tilde{m}_j = \frac{\sum_{k=1}^{j-1} F_{j-k+1/2} (\Delta m_k - \Delta \tilde{m}_k) - F_{1/2} (m_{j-1} - \tilde{m}_{j-1}) + \sum_{i=1}^j [-f_{j-i+1/2} I_i + O(h^3)]}{1 - F_{1/2}} \quad (16)$$

$$\begin{aligned} \Delta m_j - \Delta \tilde{m}_j &= \frac{\sum_{k=1}^{j-2} \Delta F_{j-k+1/2} (\Delta m_k - \Delta \tilde{m}_k) + F_{3/2} (\Delta m_{j-1} - \Delta \tilde{m}_{j-1})}{1 - F_{1/2}} \\ &\quad + \frac{\sum_{i=1}^{j-1} [-\Delta f_{j-i-1/2} I_i + O(h^3)] - f_{1/2} I_j}{1 - F_{1/2}} \quad (17) \end{aligned}$$

On the other hand, the numerical approximation of m_j is given by (6) as

$$\tilde{m}_j = \frac{F_j + \sum_{k=1}^{j-1} F_{j-k+1/2} \Delta \tilde{m}_k - F_{1/2} \tilde{m}_{j-1}}{1 - F_{1/2}},$$

Therefore, their difference is given by Equation (16), which is $O(h)$ because all the three terms in the numerator are $O(h)$. Since $\Delta m_j - \Delta \tilde{m}_j = (m_j - \tilde{m}_j) - (m_{j-1} - \tilde{m}_{j-1})$, we can have Equation (17), where $\Delta F_{j-k+1/2} = F_{j-k+1/2} - F_{j-k-1/2}$ and $\Delta f_{j-k+1/2} = f_{j-k+1/2} - f_{j-k-1/2}$. Because F and f are differentiable with bounded derivatives, $\Delta F_{j-k+1/2} = O(h)$ and $\Delta f_{j-k+1/2} = O(h)$. Therefore, all the three terms in the numerator of Equation (17) are $O(h^2)$. That is, $\Delta m_j - \Delta \tilde{m}_j = O(h^2)$. By induction, we have $\Delta m_j - \Delta \tilde{m}_j = O(h^2)$ for $j = 1, 2, \dots, n$.

Similarly, we can apply a first-order Taylor expansion to F^* and so the $m^*(t_n)$ in (4) can be expressed as

$$\begin{aligned} m^*(t_n) &= F^*(t_n) + \sum_{i=1}^n F_{n-i+1/2}^* \Delta m_i + O(h) \\ &= F^*(t_n) + \sum_{i=1}^n F_{n-i+1/2}^* (\Delta \tilde{m}_i + O(h^2)) + O(h) \\ &= F^*(t_n) + \sum_{i=1}^n F_{n-i+1/2}^* \Delta \tilde{m}_i + O(h). \end{aligned}$$

Therefore, the theorem follows. \square

A.2 Proof of Lemma 1

Express Z as $Z = \sigma U + v$, where U is the standard normal random variable. Then $aZ + b = a\sigma U + av + b$. This means that $aZ + b \sim \mathcal{N}(av + b, a^2\sigma^2)$. Therefore, according to Appendix A.2. in [30], we can have (7). On the other hand,

$$\begin{aligned} &E[e^{cZ+d} \Phi(aZ + b)] \\ &= \int_{-\infty}^{\infty} \frac{1}{\sqrt{2\pi}\sigma} \exp\left(-\frac{(z-v)^2}{2\sigma^2} + cz + d\right) \Phi(az + b) dz \\ &= \exp\left(cv + d + \frac{1}{2}c^2\sigma^2\right) \\ &\quad \times \int_{-\infty}^{\infty} \frac{1}{\sqrt{2\pi}\sigma} \exp\left(-\frac{(z-v-c\sigma^2)^2}{2\sigma^2}\right) \Phi(az + b) dz. \end{aligned}$$

When $a = 0$, the result is obvious. When $a \neq 0$, let $x = az + b$. For both $a > 0$ and $a < 0$, by applying the technique of integration by substitution, the above integral can be simplified as

$$\int_{-\infty}^{\infty} \frac{1}{\sqrt{2\pi}a\sigma} \exp\left[-\frac{(x-b-av-ac\sigma^2)^2}{2(a\sigma)^2}\right] \Phi(x) dx.$$

This is exactly the expectation of $E[\Phi(U)]$ where $U \sim \mathcal{N}(av + b + ac\sigma^2, a^2\sigma^2)$. According to (7), the integral is equal to

$$E[\Phi(U)] = \Phi\left(\frac{av + b + ac\sigma^2}{\sqrt{1 + a^2\sigma^2}}\right).$$

Combining the above results yields (8). \square

A.3 Derivation of Equation (9)

The CDF of X can be computed as

$$F_X(x) = E \left[\Phi \left(x \sqrt{\lambda/x} \frac{1}{\mu} - \sqrt{\lambda/x} \right) + \exp \left(2\lambda \frac{1}{\mu} \right) \cdot \Phi \left(-x \sqrt{\lambda/x} \frac{1}{\mu} - \sqrt{\lambda/x} \right) \right].$$

We have assumed $1/\mu \sim \mathcal{N}(v, \sigma^2)$. Therefore, the above equation can be evaluated based on Lemma 1. The result is exactly Equation (9).

A.4 Derivation of Equation (12)

The joint PDF of $\mathbf{X}_i = \{X_{i1}, X_{i2}, \dots, X_{iN_i}\}$ conditional on μ , the fixed yet unknown random effect for this system, is given by

$$f_{\mathbf{X}_i|\mu}(x_{i1}, x_{i2}, \dots, x_{iN_i}|\mu) = \left(\frac{\lambda}{2\pi} \right)^{N_i/2} \times \exp \left[-\frac{1}{2} \left(\lambda \omega_{i1} \frac{1}{\mu^2} - N_i \lambda \frac{1}{\mu} + \lambda \omega_{i2} + 3\omega_{i3} \right) \right].$$

We have assumed that $1/\mu \sim \mathcal{N}(v, \sigma^2)$. Integrating $1/\mu$ out of the above conditional joint PDF by using Lemma 2, we have that

$$f_{\mathbf{X}_i} = \left(\frac{\lambda}{2\pi} \right)^{N_i/2} (1 + \lambda \omega_{i1} \sigma^2)^{-1/2} \times \exp \left[-\frac{\lambda \omega_{i1} v^2 - 2N_i \lambda v - N_i^2 \lambda^2 \sigma^2}{2(1 + \lambda \omega_{i1} \sigma^2)} - \frac{1}{2} (\lambda \omega_{i2} + 3\omega_{i3}) \right].$$

Taking the logarithm of $f_{\mathbf{X}_i}$ and then summing over i yields the desired result.

B Data Used for Example

The following data was used for the illustrative example. Table 1 contains the CWL-to-failure for each component at each of the 15 different sites. Table 2 contains summarized data on the CWL recorded over one year for each site. Table 3 contains data on annual average turbidity as the water quality measure for each site.

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Table 1: CWL-to-failure for each RGF at each site. The number of rows corresponds to the number of components in each site.

Site	1	2	3	4	5	6	7	8
	496	674	632	813	1546	2096	1134	8215
	811	637	1215	591	868	1275	1896	4207
	652	553	965	605	1077	3943	860	1967
	585	748	1034	1115	1589	1348	1425	
	512	764	813	738	1469			
	478	561	865	784	1137			
	521	451	1120	714	1093			
	411	497	1017	641	1262			
	604	543	1113	580	810			
	482	505	1160	1145	1011			
	417	518	1040	501	942			
	768	591		932	863			
	533	285		835				
	426	436						
	674	546						
	672	476						
	664	588						
	729	432						
	657	623						
	539	397						
	795	541						
	673							
Site	9	10	11	12	13	14	15	
	11991	1473	10268	1639	1219	1004	9066	
	5674	897						
	5339							

Table 2: CWL for each site over a one year period.

Site	1	2	3	4	5
Mean monthly CWL	134.75	68.42	136.83	121.42	5.83
Annual CWL	1617	821	1642	257	430
Site	6	7	8	9	10
Mean monthly CWL	36.83	28.83	64	39.83	37.83
Annual CWL	442	346	768	478	454
Site	11	12	13	14	15
Mean monthly CWL	35.42	13.08	20.75	18.33	18.25
Annual CWL	425	157	249	220	219

Table 3: Average turbidity data for each site.

Site	1	2	3	4	5	6	7	8
Turbidity	1.4	1.53	1.04	1.2	0.9	0.6	0.7	0.4
Site	9	10	11	12	13	14	15	
Turbidity	0.3	0.8	0.2	0.9	0.8	0.7	0.3	

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